



Riverside MS FEI

100% Bid Documents

East Providence, RI

Ai3 Project # - 1903.03

Addendum #1

September 25, 2023

The attention of Bidders submitting proposals for the Riverside MS FEI 100% Bid Documents is called to the following changes to the Bidding Contract Documents dated September 18, 2023 as prepared by Ai3 Architects, LLC. The items set forth therein below, whether of revision, omission, addition, substitution or clarification are all to be included as changes to Information to Bidders, the Conditions of the Contract, Specifications and Drawings of the Contract.

The number of this Addendum (Number 1) must be entered in the appropriate spaces provided on the Bid Form.

ATTACHMENTS:

ADD 1-001 Contractor Prebid Conference September 21, 2023 walk through sign in sheet.

Structural:

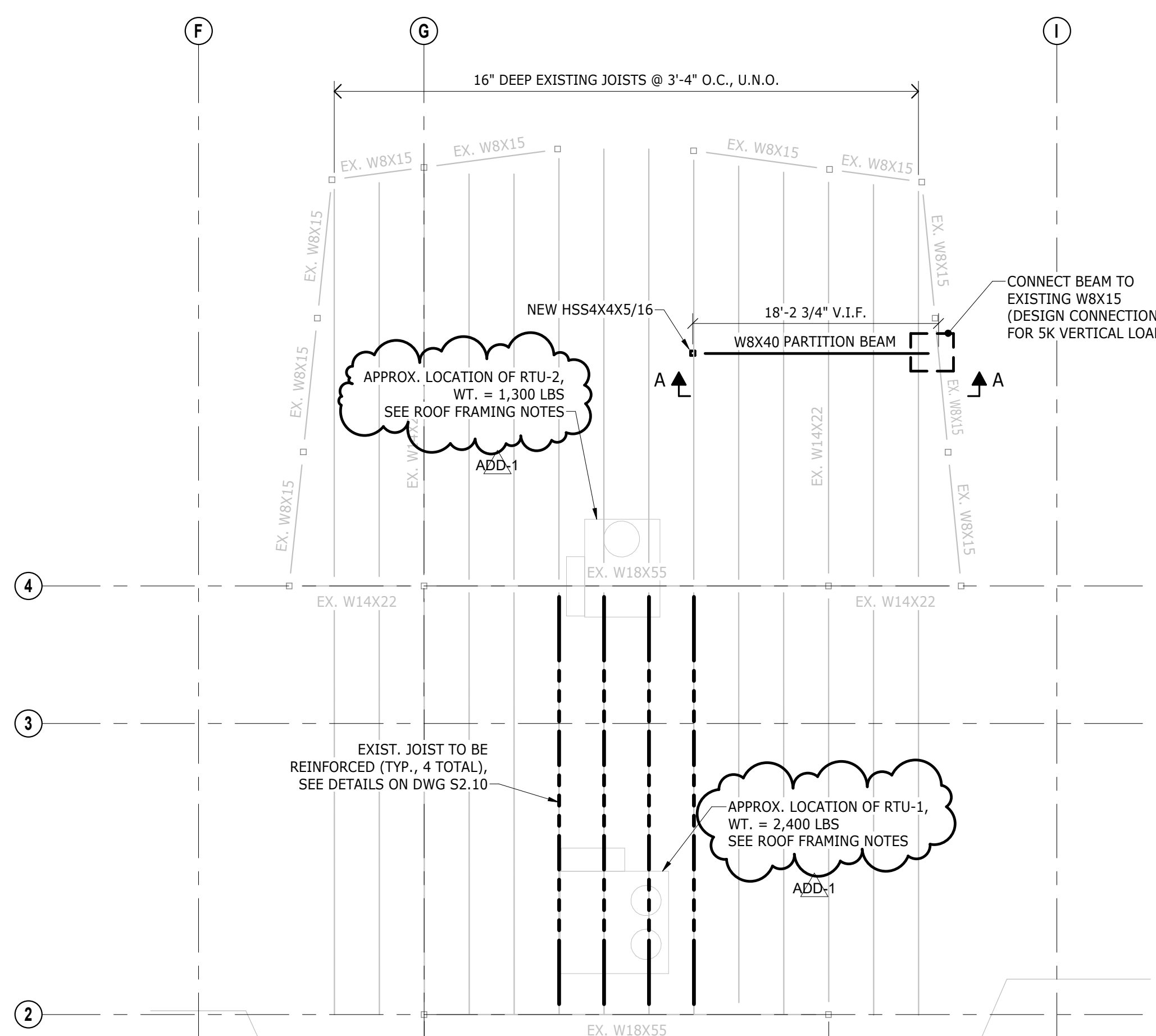
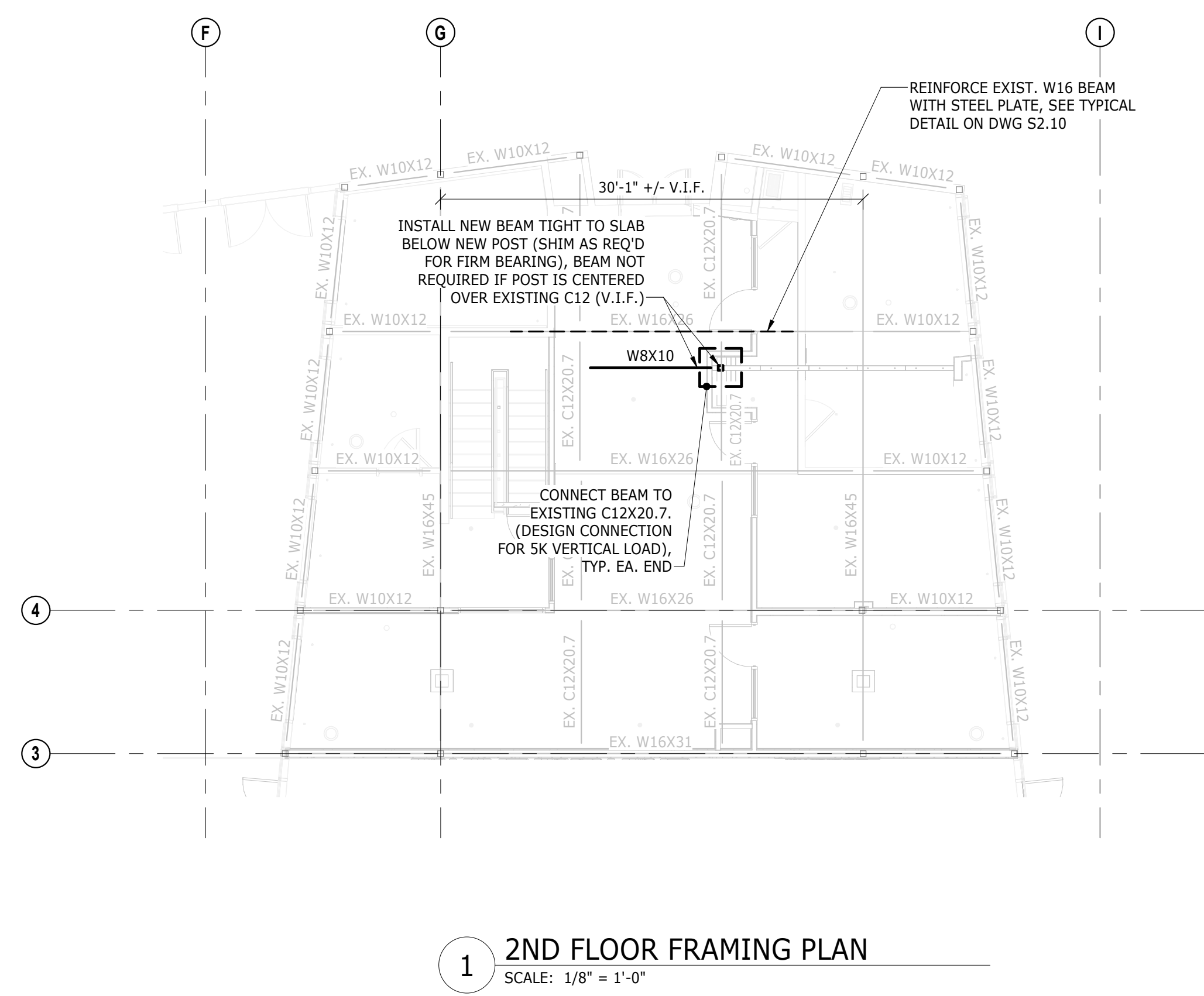
ADD 1-002 Structural Drawings: REMOVE drawings S1.10 and S2.10 in their entirety and replace with drawings S1.10 and S2.10 bound here within Addendum #1.

END OF ADDENDUM #1

Riverside MS FEI
100% BID DOCUMENTS
EAST PROVIDENCE, RI
ADD #1 – PAGE 1

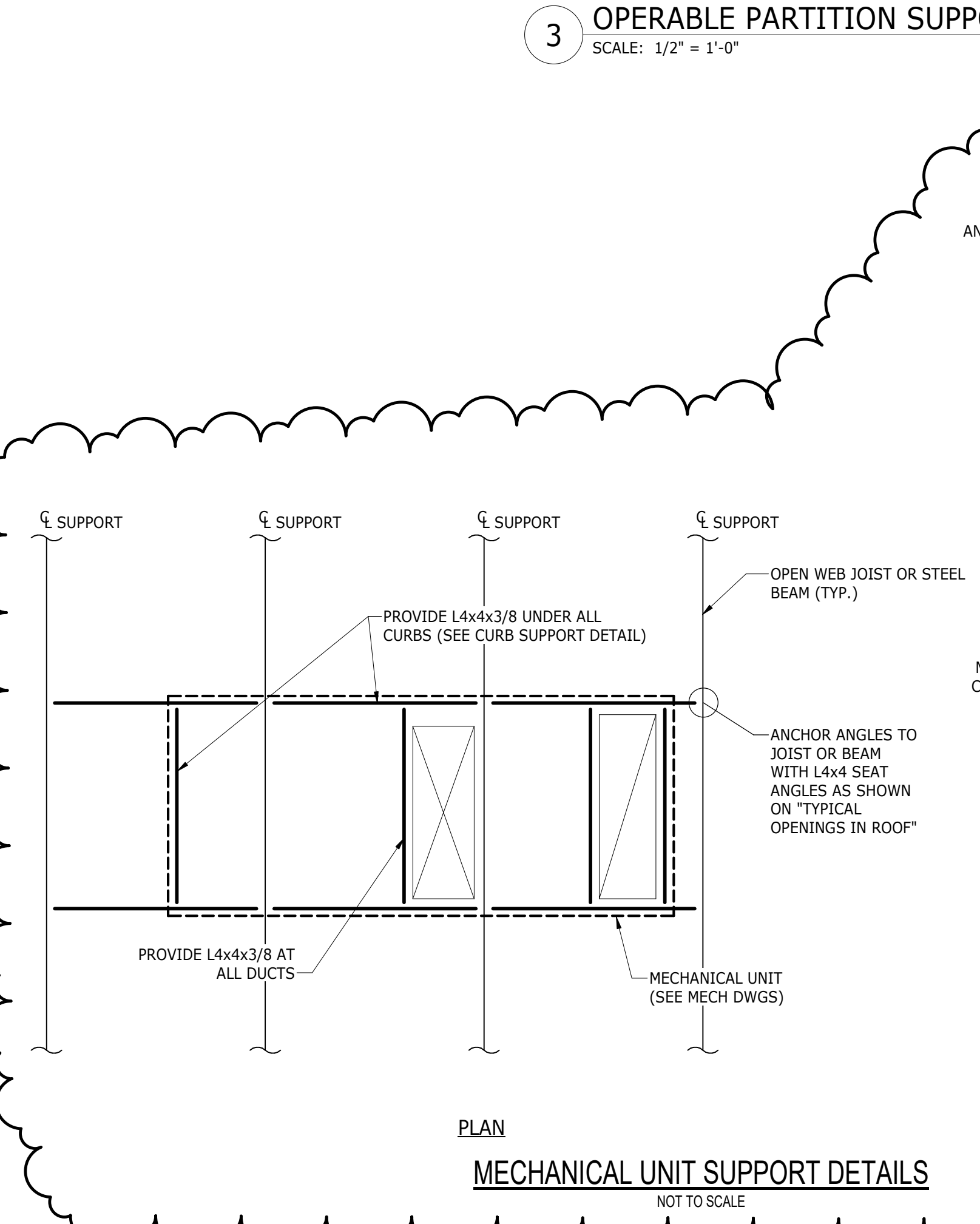
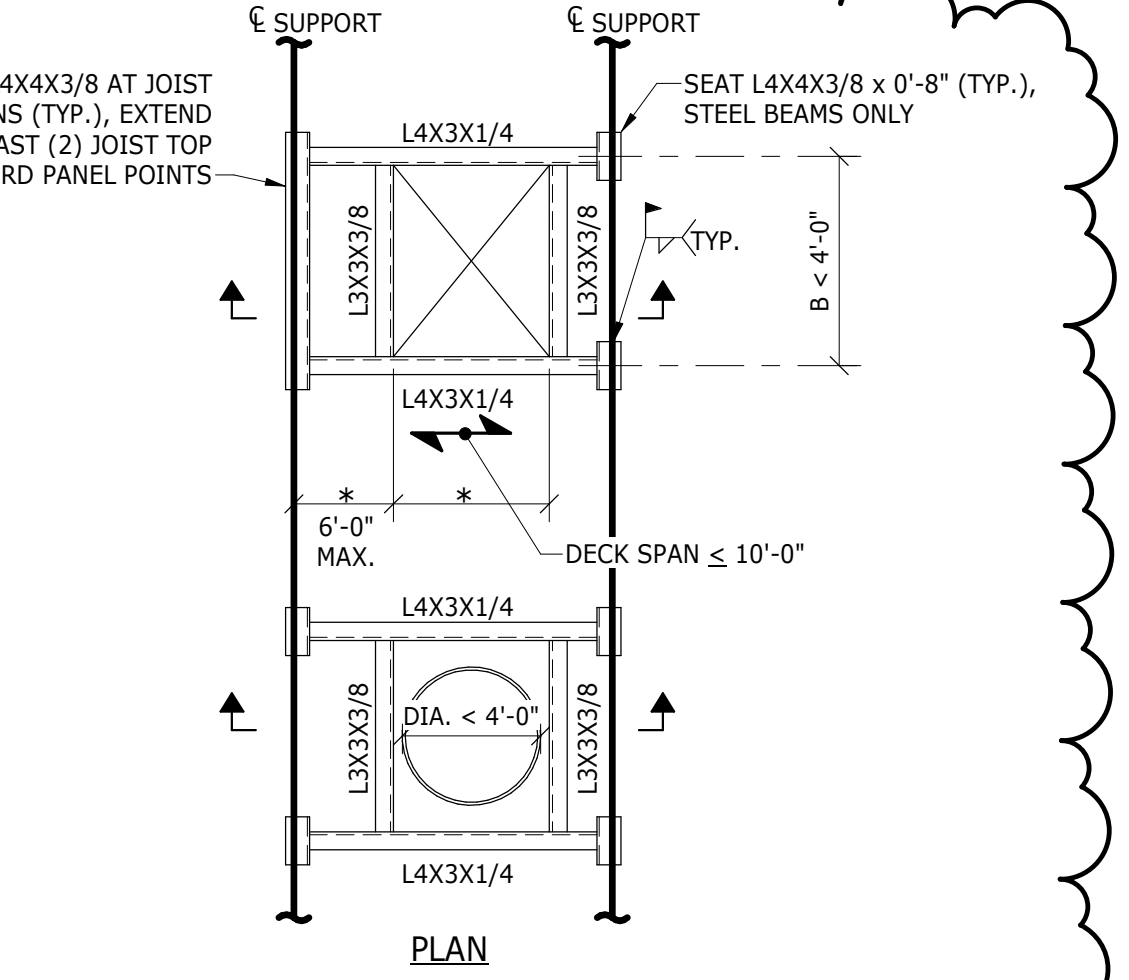
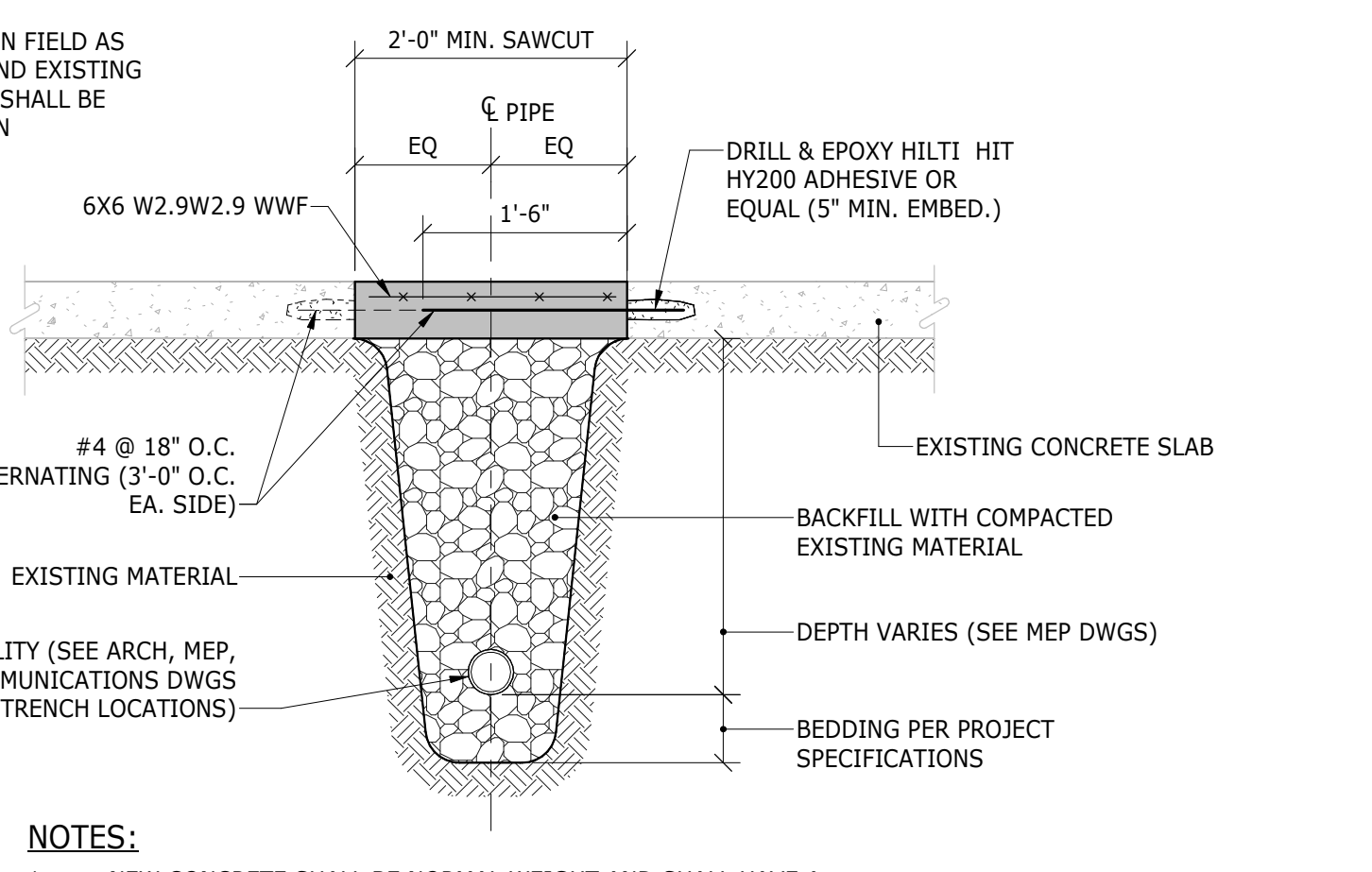
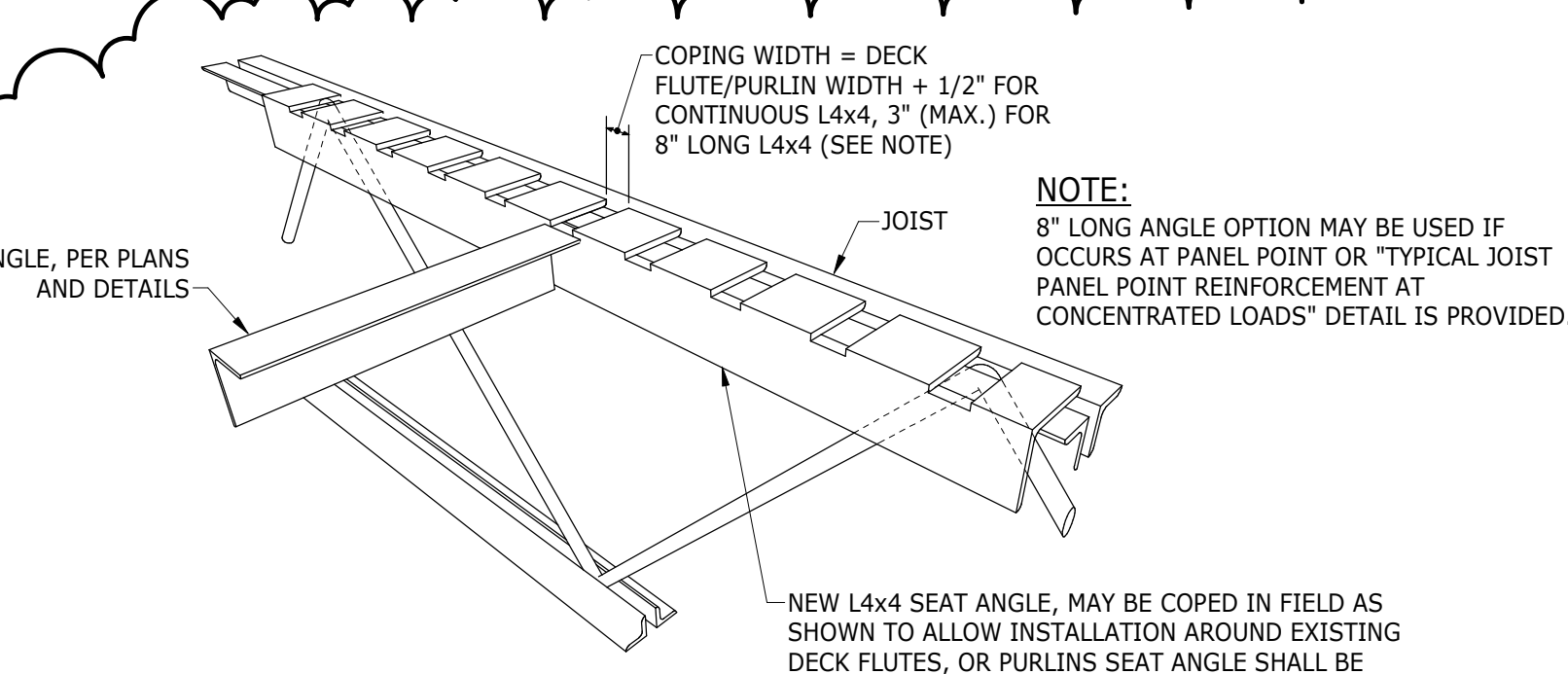
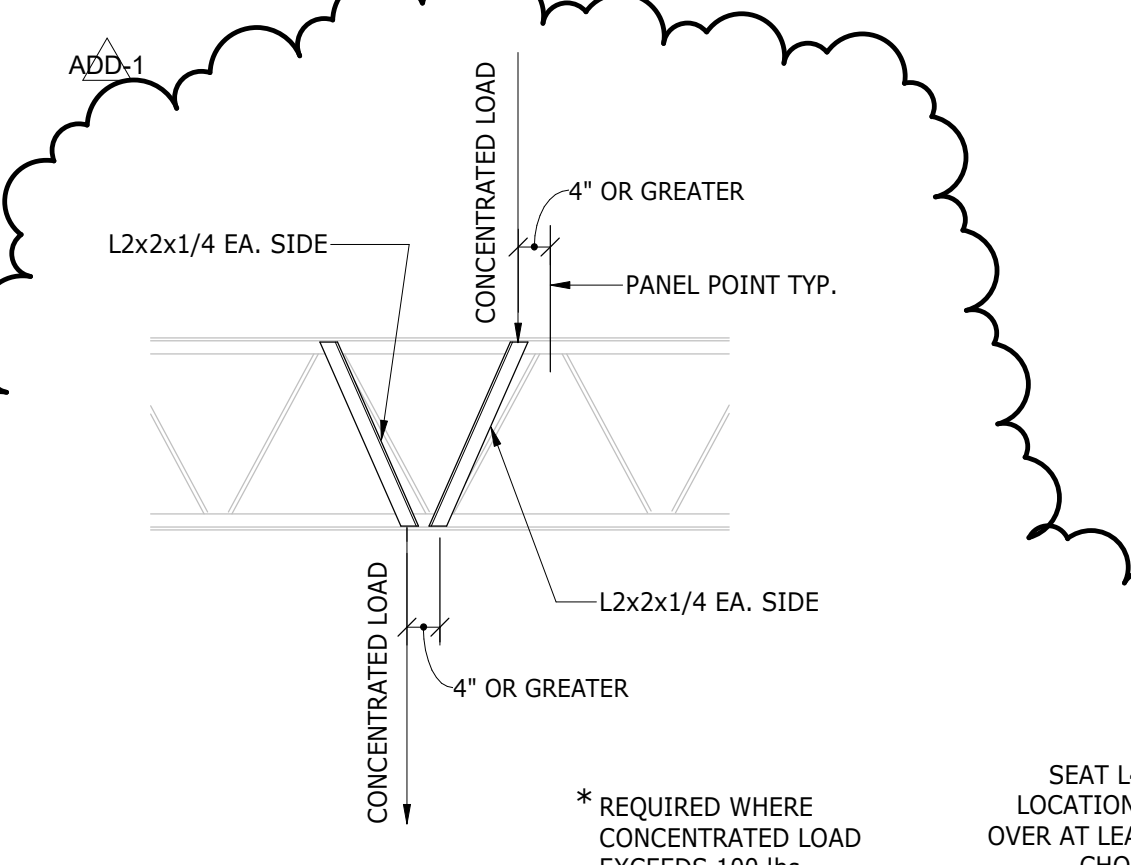
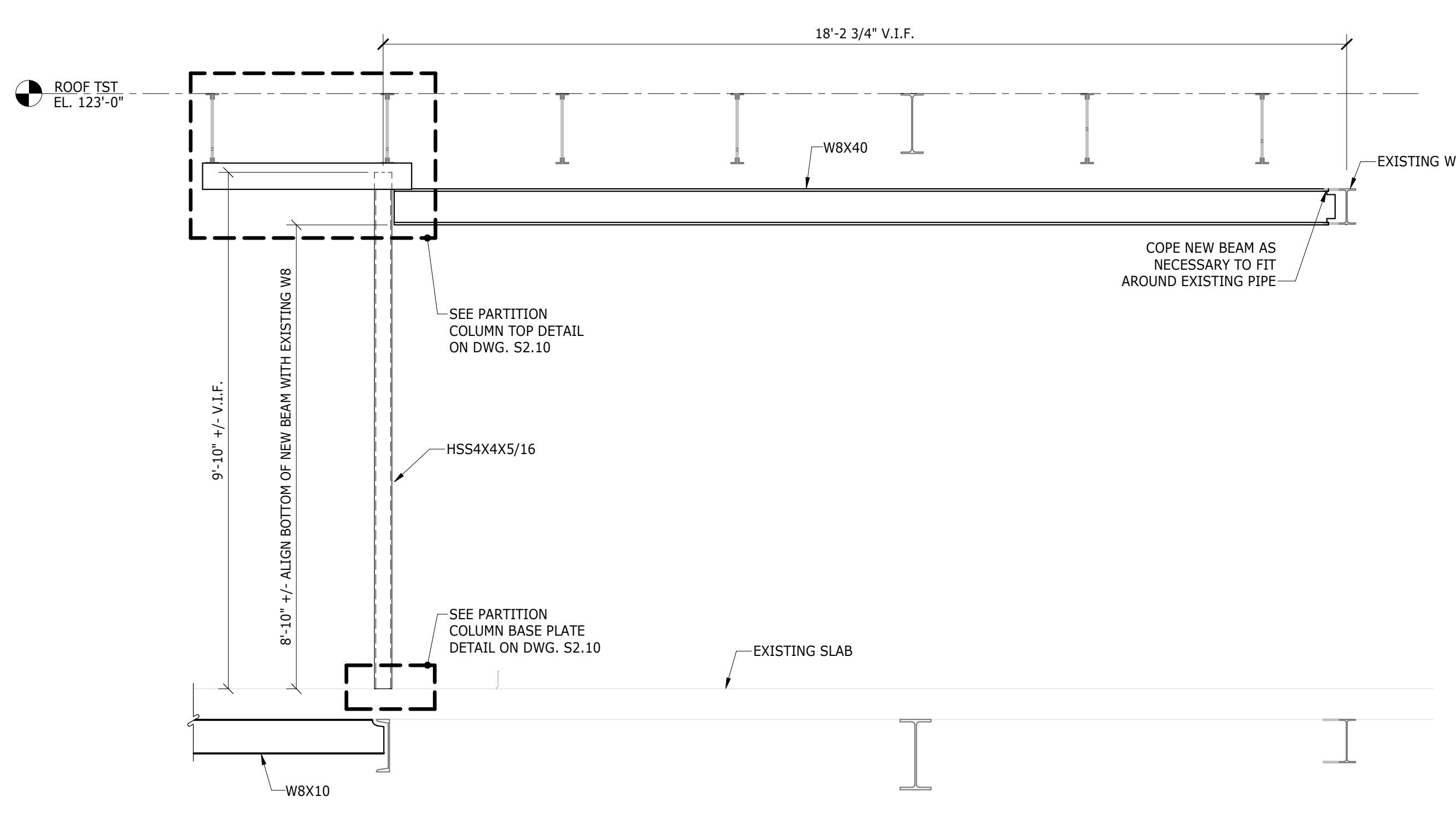
Riverside Middle School
September 21, 2023

Name	Company	Email	Phone
Jareed Behm	Excell Construction	jbehm@excellri.com	(978) 578-5284
Joe Brooks	Excell Construction	jbrooks@excellri.com	()
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Shawn Viveiros	East Bay Power LLC	sviveiros@eastbaypower.com	(401) 447-5884
Ashley DaCunha	Collins Construction Co. Inc	ashley@collinsconst.net	508-678-5201
Matthew DePasquale	ADS Construction	Matt@adsconst.com	(401) 465-1452
Joseph Ptaszek	Ptaszek Construction	JOEPTASZ@AOL.COM	(401) 524-2498
WILLIAM HUNTER	MARON CONSTRUCTION	WHUNTER@MARONCO.COM	401-272-4950
DERRICK PAPA	TOWER	estimating@towerconstructioncorp.com	401.943.0110



ROOF FRAMING NOTES:

- NOTE THAT RELOCATION, REMOVAL, AND/OR REPLACEMENT OF EXISTING CONSTRUCTION/UTILITIES MAY BE REQUIRED FOR THE WORK. CONTRACTOR SHALL REVIEW SITE CONDITIONS PRIOR TO SUBMITTING A BID.
- REMOVE AND RESET EXISTING JOIST BRIDGING AS REQUIRED TO COMPLETE THE WORK.
- PER THE STEEL JOIST INSTITUTE (SJI), TEMPORARY SUPPORT/SHORING OF JOISTS TO BE REINFORCED IS RECOMMENDED AS WELDING HEAT CAN TEMPORARILY WEAKEN STRUCTURAL COMPONENTS.
- G.C. COORDINATE ROOFTOP HVAC LOCATIONS AND DIMENSIONS. VERIFY UNIT WEIGHTS DO NOT EXCEED WEIGHTS AS SHOWN ON PLANS. FINAL LOCATION SHALL BE FIELD VERIFIED BASED ON ACTUAL RTU DIMENSIONS. PROVIDE FRAMING BELOW ALL CURBS PER TYPICAL DETAILS.
- GENERAL CONTRACTOR SHALL COORDINATE LOCATION OF ALL OPENINGS WITH ARCHITECTURAL AND MEP DRAWINGS. FRAME NEW OPENINGS PER TYPICAL DETAILS.



ABBREVIATIONS

ADD'L	ADDITIONAL	LG.	LONG
ALT	ALTERNATE	LONG.	LONGITUDINAL
A.B.	ANCHOR BOLT	LV.	LONG LEG VERTICAL
ARCH	ARCHITECT	L.W.	LONG WAY
BOT.	BOTTOM	L.W.C.	LIGHT WEIGHT CONCRETE
BEW	BOTTOM EACH WAY	MAX.	MAXIMUM
BM	BEAM	MECH	MECHANICAL
BOF	BOTTOM OF FOOTING	M.M.	MISCELLANEOUS METAL
BRG	BEARING	MIN.	MINIMUM
BS	BOTH SIDES	MTL	METAL
C	CAMBER	NF	NEAR FACE
CFMF	COLD FORMED METAL FRAMING	NS	NON SHRINK
CIP	CAST-IN-PLACE	NTS	NOT TO SCALE
CLR	CLEAR	O.C.	ON CENTER
COL	COLUMN	PL	PLATE
COMP. DK.	COMPOSITE DECK	OPNG	OPENING
CONC.	CONCRETE	R & D	REMOVE AND DISPOSE
CMU	CONCRETE MASONRY UNIT	REINF.	REINFORCING
CJ	CONTROL JOINT	SC	SHOAR CONNECTOR
CONST. JT.	CONSTRUCTION JOINT	SLV	SHORT LEG VERTICAL
CONT.	CONTINUOUS	SOG	SLAB ON GRADE
DIA or Ø	DIAMETER	S.S.	STAINLESS STEEL
DWLS	DOWELS	STIFF	STIFFENER
DWG	DRAWING	STL	STEEL
EA.	EACH	T	TOP
E.F.	EACH FACE	TOP	TOP
E.W.	EACH WAY	TCX	TOP CHORD EXTENSION
EL.	ELEVATION	THK	THICK
E.J.	EXPANSION JOINT	TOC	TOP OF CONCRETE
EQ.	EQUAL	TOP	TOP OF FOOTING
EX. or EXIST.	EXISTING	TOW	TOP OF WALL
F.F.	FAR FACE	TRANS.	TRANSVERSE
F.FE	FINISH FLOOR ELEVATION	TOP OF SLAB	TOP OF SLAB
FND	FOUNDATION	TST	TOP OF STEEL
FTG	FOOTING	TRNS.	TRANSVERSE
GA.	GAUGE	U.N.O.	UNLESS NOTED OTHERWISE
GALV.	GALVANIZED	VERT.	VERTICAL
G.C.	GENERAL CONTRACTOR	V.I.F.	VERIFY IN FIELD
HORIZ.	HORIZONTAL	WWF	WELDED WIRE FABRIC
HSS	HOLLOW STRUCTURAL SHAPE	W	WITH
I.F.	INSIDE FACE	W.P.	WORKING POINT
JBE	JOIST BEARING ELEVATION		

A. GENERAL STRUCTURAL REQUIREMENTS

- ALL METHODS OF CONSTRUCTION, DETAILS, NOTES, ETC., INDICATED ON THE DRAWINGS ARE TO BE CONSIDERED TYPICAL FOR ALL SIMILAR CONDITIONS.
- CONSTRUCTION SHALL BE MADE FROM APPROVED SHOP DRAWINGS ONLY.
- ANY DISCREPANCIES ON THESE PLANS WITH REGARD TO DIMENSIONS OR CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PORTION OF WORK.
- ALL APPLICABLE FEDERAL, STATE, AND MUNICIPAL REGULATIONS SHALL BE FOLLOWED, INCLUDING THE FEDERAL DEPARTMENT OF LABOR OCCUPATIONAL SAFETY AND HEALTH ACT AND THE RHODE ISLAND STATE BUILDING CODE.
- THE LATEST EDITION OF THE FOLLOWING LISTED CODES SHALL APPLY. IN CASE OF CONFLICT, THE MORE RIGID REQUIREMENTS AND CODES SHALL GOVERN.
 - RHODE ISLAND STATE BUILDING CODE (STATE CODE); INTERNATIONAL BUILDING CODE, 2018 EDITION AND ITS APPLICABLE REFERENCED STANDARDS.
 - AMERICAN INSTITUTE OF STEEL CONSTRUCTION SPECIFICATIONS AND ITS CODE OF STANDARD PRACTICE (AISC).
 - AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, ACI 318.
 - AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES, ACI 530 AND ACI 530.1.
- THE DESIGN LOADS ARE RESISTED BY THE COMPLETED STRUCTURE ACTING AS A UNIT. THE CONTRACTOR SHALL DESIGN AND PROVIDE ANY AND ALL TEMPORARY BRACING, SHORING, OR ADDITIONAL REINFORCEMENT NECESSARY TO RESIST LOADS IMPOSED ON ANY PORTION OF THE STRUCTURE THROUGHOUT ALL STAGES OF CONSTRUCTION. THE SHORING SHALL BE DESIGNED TO RESIST ALL DEAD LOADS AND ANY APPLICABLE CONSTRUCTION LOADS.
- ALL SHORING DESIGNS AND PLANS SHALL BE STAMPED BY A RHODE ISLAND REGISTERED PROFESSIONAL ENGINEER.
- COLUMN ANCHOR RODS ARE NOT DESIGNED TO TEMPORARILY CANTILEVER FROM THE FOUNDATIONS. ALL STEEL MUST BE TEMPORARILY BRACED AND GUYED UNTIL THE BUILDING IS LATERALLY STABLE.
- NOTES AND TYPICAL DETAILS APPLY TO ALL STRUCTURAL WORK UNLESS OTHERWISE NOTED. FOR CONDITIONS NOT SPECIFICALLY SHOWN PROVIDE DETAILS OF SIMILAR NATURE. VERIFY APPLICABILITY BY SUBMITTING SHOP DRAWINGS FOR REVIEW.
- PLANS SHALL NOT BE SCALED FOR DIMENSIONS.
- ARCHITECTURAL AND MEP DRAWINGS MUST BE USED IN CONJUNCTION WITH THE STRUCTURAL DRAWINGS DURING ALL PHASES OF CONSTRUCTION.

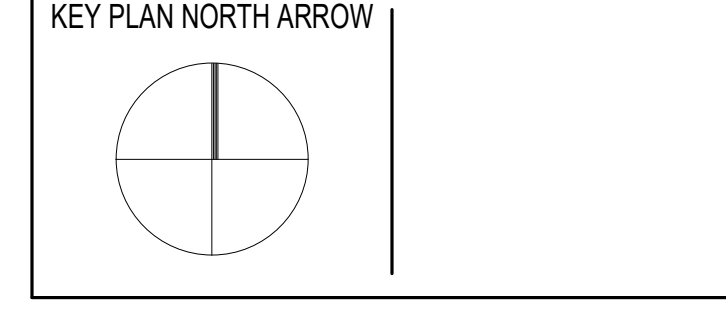
B. STRUCTURAL STEEL

- DESIGN FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH AISC SPECIFICATION FOR BUILDINGS.
- NEW STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING:

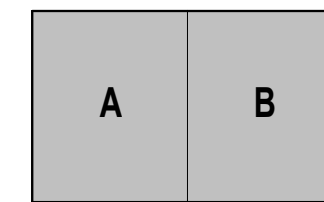
A. STRUCTURAL STEEL	A572 OR A992 GR. 50	Fy=50 KSI
B. TYPICAL PLATES AND ANGLES	ASTM A36	Fy=36 KSI
C. STRUCTURAL TUBING	ASTM A500, GR. B	Fy=46 KSI
		Fy=42 ksi (ROUND)
D. HIGH STRENGTH BOLTS	ASTM F3125 (GR. A325 TYPE 1)	Fy=92 KSI
E. CAST-IN-PLACE ANCHOR RODS	F1554 (GRADE 36)	Fy=36 KSI
F. HEADED STUDS	A108 GR. 50	Fy=50 KSI
G. DRILL & EPOXY ANCHOR RODS	A449	Fy=92 KSI
- SHAPES NOTED "GALV." ON DRAWINGS SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A123.
- ALL STRUCTURAL STEEL CONNECTIONS NOT SPECIFICALLY DETAILED ON THE PLANS SHALL BE DESIGNED BY THE CONTRACTOR IN ACCORDANCE WITH THE CURRENT EDITION OF THE AISC "MANUAL OF STEEL CONSTRUCTION - ALLOWABLE STRENGTH DESIGN (ASD)". DESIGN FOR ALL CONNECTIONS SHALL BE STAMPED BY A RHODE ISLAND PROFESSIONAL ENGINEER ENGAGED BY THE CONTRACTOR AND SUBMITTED TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO FABRICATION. CONNECTIONS SHALL BE DESIGNED TO DEVELOP (1/2) OF MEMBER'S TOTAL UNIFORM LOAD CAPACITY, TYPICAL UNLESS NOTED OTHERWISE.
- ALL BOLTED CONNECTIONS SHALL USE 3/4" DIA., A-325-N TYPE 1 BOLTS, UNLESS NOTED OTHERWISE.
- ALL NEW STRUCTURAL STEEL SHALL BE GIVEN ONE COAT OF AN APPROVED SHOP PRIMER AND PAINT APPLIED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS, UNLESS NOTED OTHERWISE (SEE NOTE 8 BELOW). DO NOT PAINT TOP FLANGES OF BEAMS THAT RECEIVE SHEAR STUDS. SHOP PAINTING OF STRUCTURAL STEEL SHALL CONFORM TO SSPC-SP2 (INTERIOR SURFACES) OR SSPC-SP6 (EXTERIOR SURFACES).
- AFTER ERECTION IS COMPLETE, TOUCH-UP ALL SHOP PRIMED COATS DAMAGED DURING TRANSPORT AND ERECTION, AND PRIME ALL FIELD WELDS USING THE SAME PAINT USED FOR SHOP PRIMING.
- ANY STRUCTURAL STEEL TO RECEIVE SPRAY-ON FIREPROOFING SHALL BE FABRICATED WITHOUT ANY PRIMER OR PAINT COATINGS. COORDINATE WITH THE ARCHITECT REGARDING ADDITIONAL INFORMATION RELATED TO FIREPROOFING.
- ALL EXPOSED STRUCTURAL STEEL AND CONNECTORS SHALL BE PRIMED AND PAINTED WITH AN APPROVED PAINT SYSTEM. COORDINATE REQUIREMENTS WITH THE ARCHITECT.
- HIGH STRENGTH BOLTS SHALL BE TORQUED TO 70% OF THE MINIMUM TENSILE STRENGTH OF THE BOLT IN CONFORMANCE WITH AISC SPECIFICATION FOR STRUCTURAL JOINTS USING A-325-N TYPE 1 BOLTS. PROVIDE ONE HARDENED WASHER UNDER THE ELEMENT TURNED IN TIGHTENING.
- WELDS SHALL BE MADE ONLY BY OPERATORS CERTIFIED BY THE STANDARD QUALIFICATION PROCEDURE OF THE AMERICAN WELDING SOCIETY. TOUCH UP ALL WELDS WITH THE APPROVED PAINT SYSTEM.
- WELDING: IN ACCORDANCE WITH LATEST EDITION OF AWS D1.1 CODE FOR WELDING IN BUILDING CONSTRUCTION. USE E70 SERIES ELECTRODES UNLESS NOTED OTHERWISE.
- FIELD WELDING OF STRUCTURAL MEMBERS IS NOT PERMITTED UNLESS SPECIFICALLY INDICATED.
- FURNISH AND INSTALL ONE WASHER AND ONE HEAVY HEX NUT WITH ASTM F1554 ANCHOR BOLTS UNLESS OTHERWISE INDICATED.
- PROVIDE FITTED WELDED 3/8" WEB STIFFENER PLATES ON EACH SIDE OF ALL BEAMS SEATED ON WALLS OR COLUMNS UNLESS NOTED OTHERWISE.
- FIELD CUTTING OR MODIFICATION OF STRUCTURAL STEEL IS PROHIBITED UNLESS PRIOR WRITTEN APPROVAL IS RECEIVED FROM THE ENGINEER.
- SURFACES OF GALVANIZED MEMBERS TO BE WELDED SHALL BE GROUND TO BARE METAL PRIOR TO WELDING, AND TOUCHED UP AFTER WELDING IN ACCORDANCE WITH PROJECT SPECIFICATIONS.
- MINIMUM FILLET WELD (LEG) SIZE SHALL BE 3/16", UNLESS NOTED OTHERWISE.
- SHEARED ENDS OF GALVANIZED PRETENSIONED TWIST-OFF SPLINE BOLTS SHALL BE TOUCHED UP WITH A ZINC RICH PRIMER IN ACCORDANCE WITH ASTM A780 AFTER INSTALLATION.

ADDENDUM #1 ADD-1 9.25.2023

ADDENDUMIZED DOCUMENTS



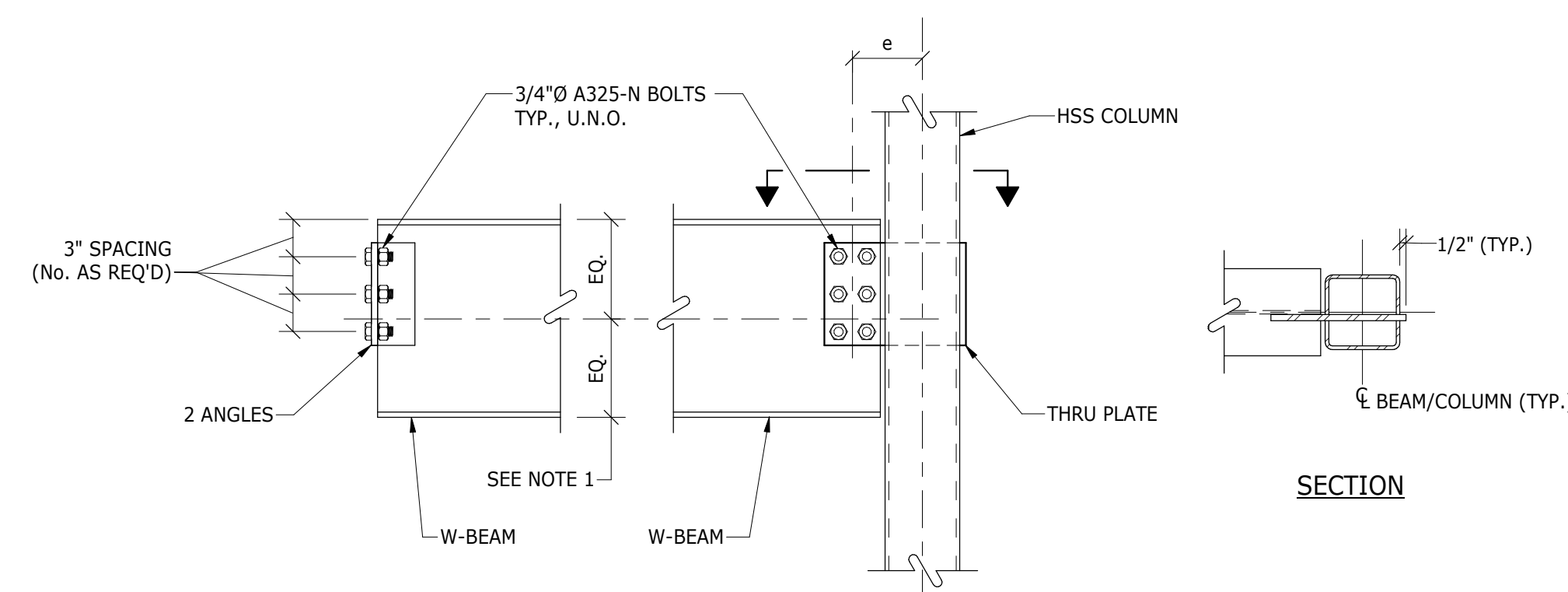
KEYPLAN



DRAWING NAME:

FRAMING PLANS, NOTES, AND ELEVATION

DRAWN BY: MSS
REVIEWED BY: KMC
SCALE: AS INDICATED | DRAWING NUMBER:
JOB NO.: 1903.03
DATE: September 18, 2023 **S1.10**



CONNECTION DESIGN FORCES (UNLESS OTHERWISE NOTED ON FRAMING PLANS)			
NON-COMPOSITE BEAMS		COMPOSITE BEAMS	
BEAM DEPTH	DESIGN AND REACTION	BEAM DEPTH	DESIGN AND REACTION
ALL BEAMS	0.50 x R	W8 TO W18	1.00 x R
		≥W21	0.75 x R

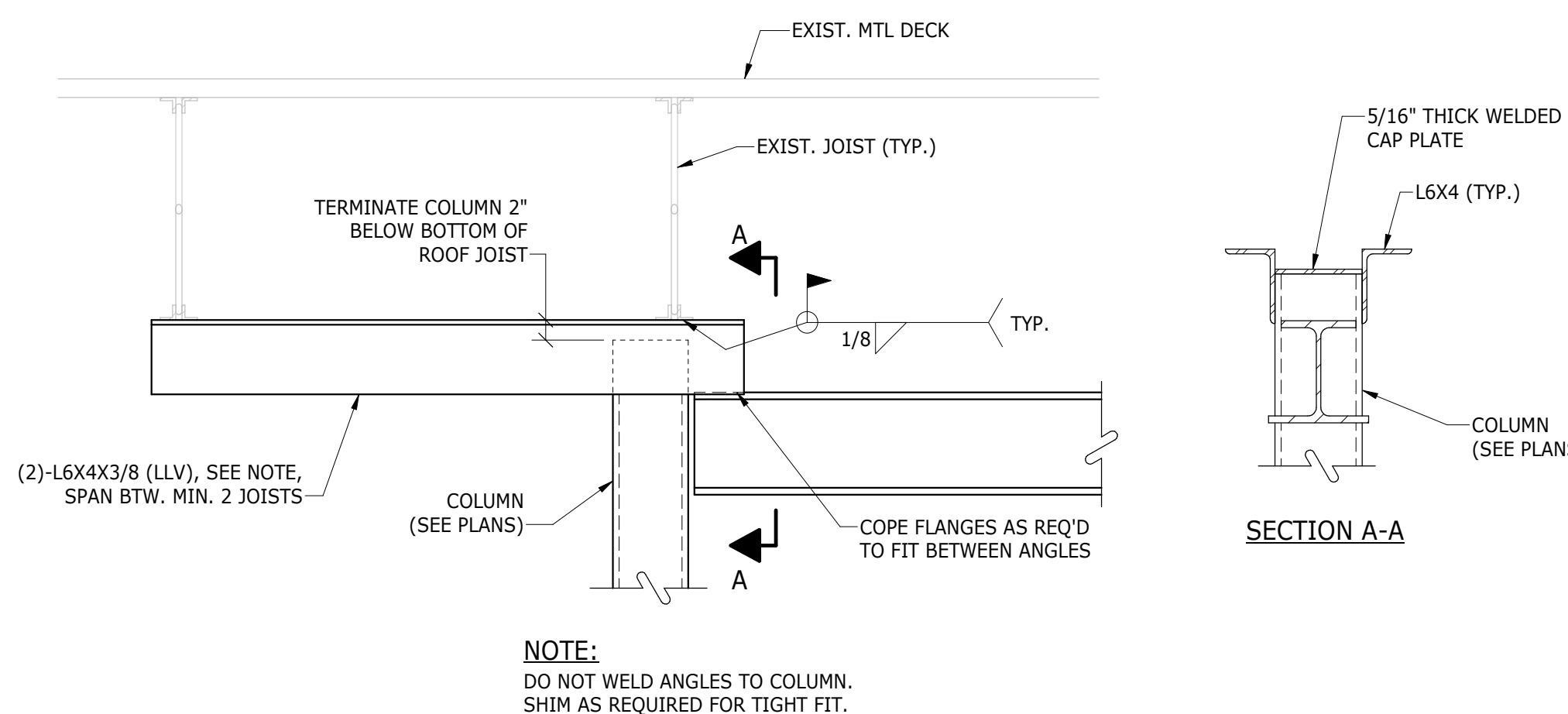
R = THE TOTAL UNIFORM LOAD CAPACITY SHOWN IN THE "MAXIMUM TOTAL UNIFORM LOAD TABLES" (ASD), PART 3 OF THE AISC MANUAL OF STEEL CONSTRUCTION FOR THE GIVEN BEAM SIZE, SPAN, AND GRADE OF STEEL UNLESS OTHERWISE NOTED ON THE PLANS.

NOTES:

- SPACING OF BOLTS SHALL EXTEND AT LEAST TO THE MID-DEPTH OF THE BEAM.
- SECONDARY W8, W10 AND W12 BEAMS NEED NOT APPLY TO THE REQUIREMENTS OF TABLE ABOVE.
- CONNECTIONS SHALL CONFORM TO THE REQUIREMENTS OF PART 4, "CONNECTIONS" OF THE AISC MANUAL OF STEEL CONSTRUCTION.
- ALTERNATE BEAM CONNECTIONS MAY BE SUBMITTED BY THE STRUCTURAL STEEL FABRICATOR FOR CONSIDERATION BY THE STRUCTURAL ENGINEER.
- BOLTS FOR THRU PLATE CONNECTIONS SHALL BE DESIGNED FOR THE ECCENTRICITY *e*.

MINIMUM REQUIREMENTS FOR BEAM CONNECTIONS

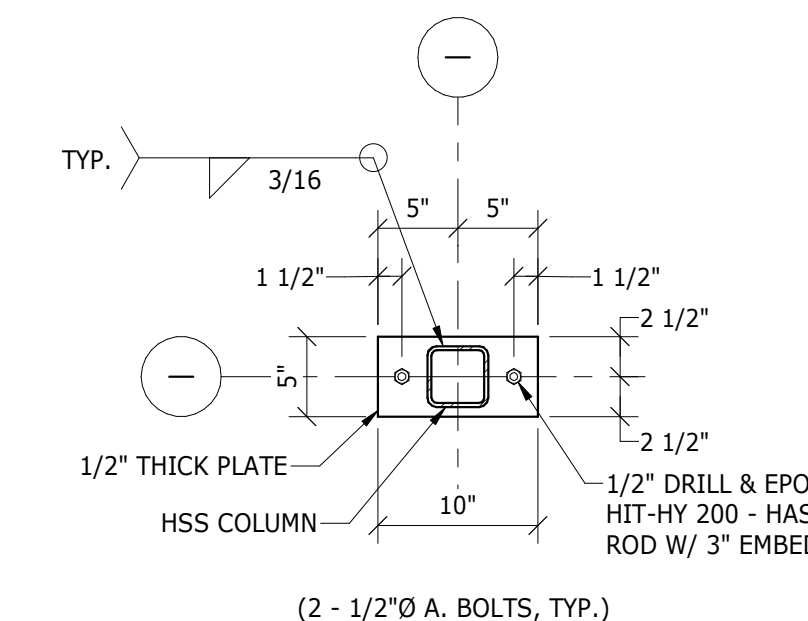
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NOTE:
DO NOT WELD ANGLES TO COLUMN. SHIM AS REQUIRED FOR TIGHT FIT.

OPERABLE PARTITION COLUMN TOP DETAIL

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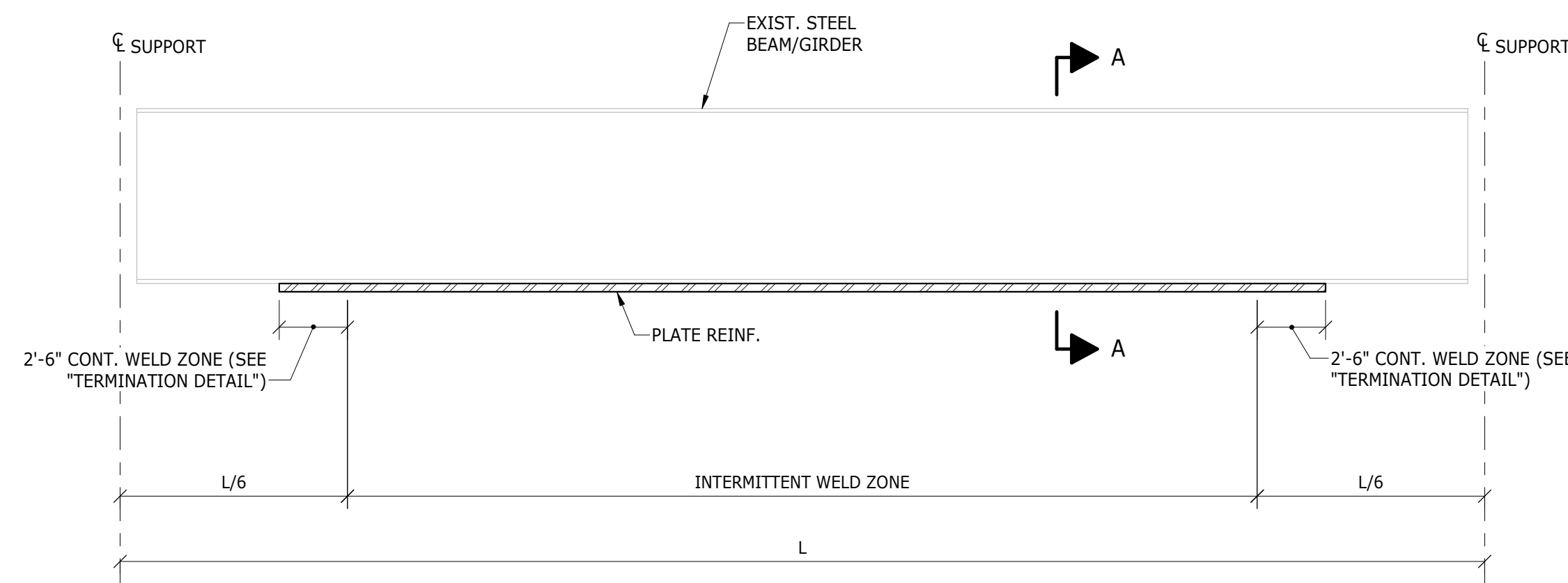


OPERABLE PARTITION COLUMN BASE PLATE DETAIL

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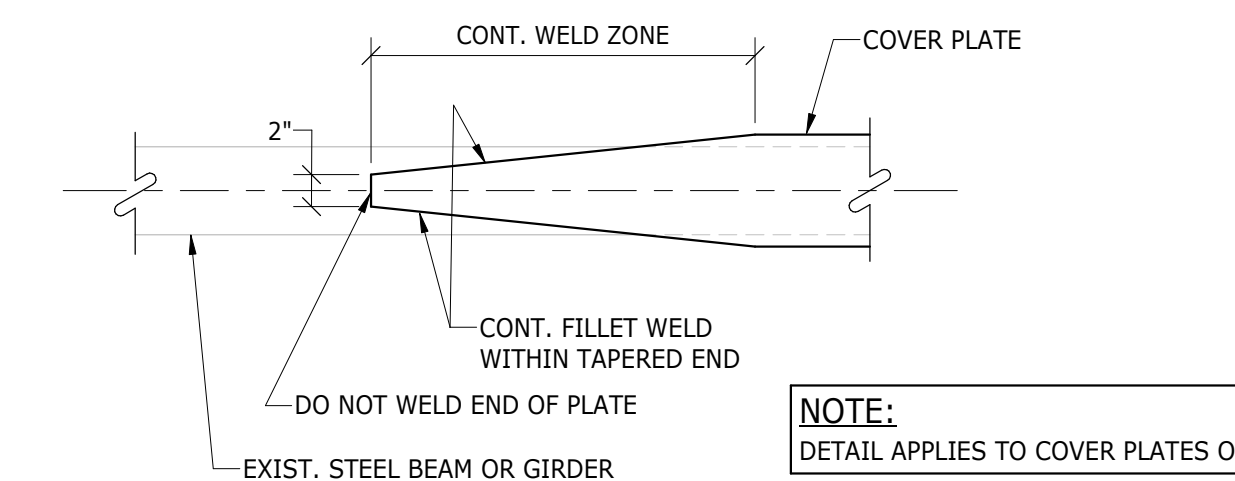
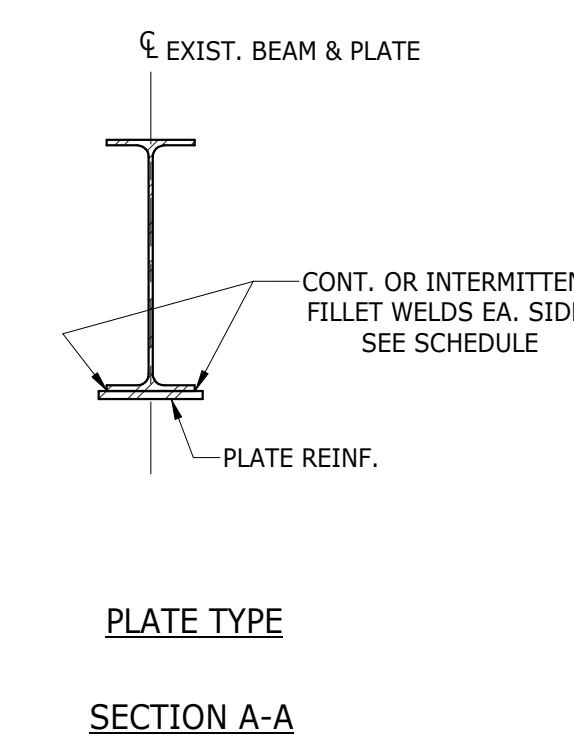
REINFORCEMENT SCHEDULE			
EXIST. BEAM SIZE	REINF. SIZE	CONT. WELD SIZE	INTERMITTENT WELD SIZE/ SPACING
W16X26	# 6 1/2"x7/16"	3/16"	3/16" - 3"@12"

NOTE:
COVER PLATES SHALL HAVE A MINIMUM YIELD STRENGTH (F_y) OF 50 ksi.



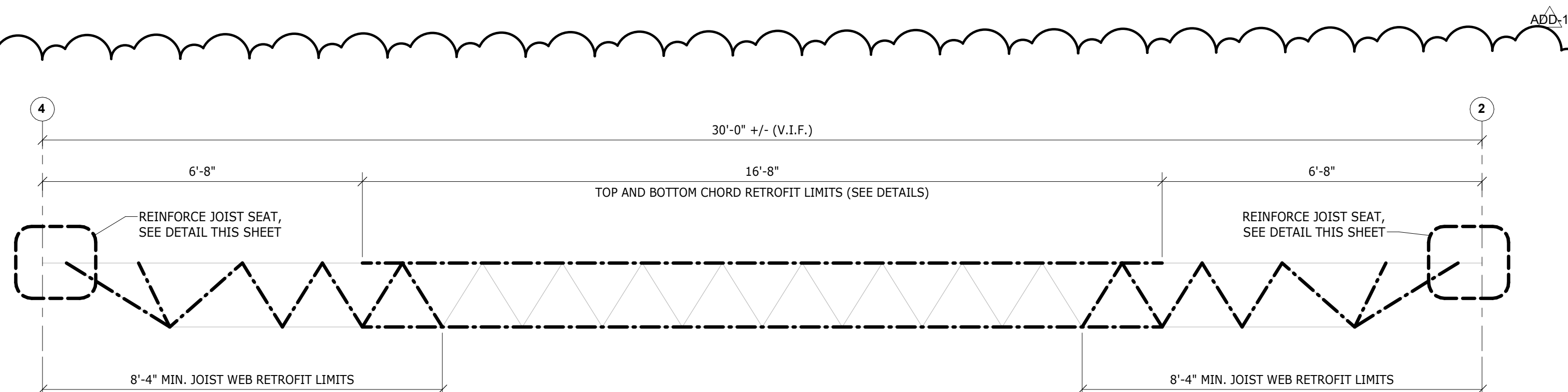
EXISTING STEEL BEAM RETROFIT DETAIL

NOT TO SCALE



TERMINATION DETAIL

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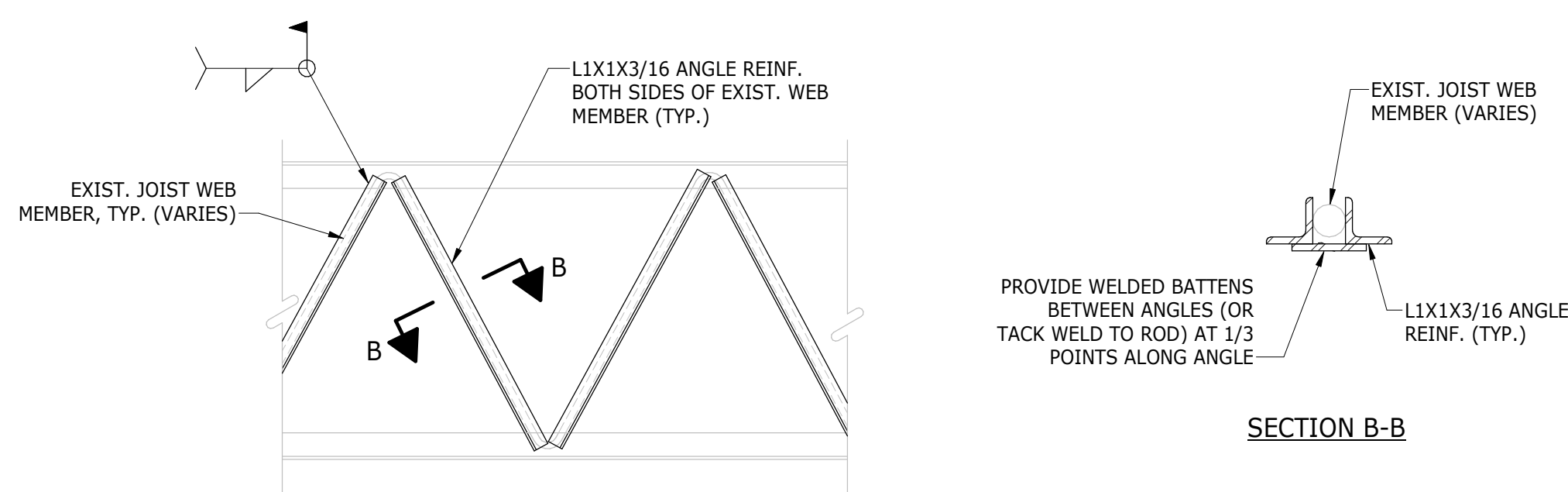


NOTES:

- PER THE STEEL JOIST INSTITUTE (SJI), TEMPORARY SUPPORT/SHORING OF JOIST TO BE REINFORCED IS RECOMMENDED AS WELDING HEAT CAN TEMPORARILY WEAKEN THE JOIST GIRDER.
- WEB MEMBERS SHOWN SCHEMATICALLY FOR THE REINFORCEMENT INTENT ONLY. VERIFY QUANTITY OF WEB MEMBERS WITHIN THE "WEB RETROFIT LIMITS" DIMENSION IN FIELD. ALL WEB MEMBERS WITHIN "WEB RETROFIT LIMITS" SHALL BE REINFORCED PER DETAILS THIS SHEET.
- ALL REINFORCEMENT COMPONENTS SHALL BE CONTINUOUS. SECTIONS OF CHORD REINFORCEMENT COMPONENTS MAY BE SPLICED TOGETHER IF REQUIRED USING A GROOVE WELD THAT DEVELOPS THE FULL STRENGTH OF THE COMPONENT. SUBMIT DETAIL TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL.
- INDICATES MEMBER TO BE REINFORCED

EXISTING ROOF JOIST REINFORCING DETAIL

NOT TO SCALE

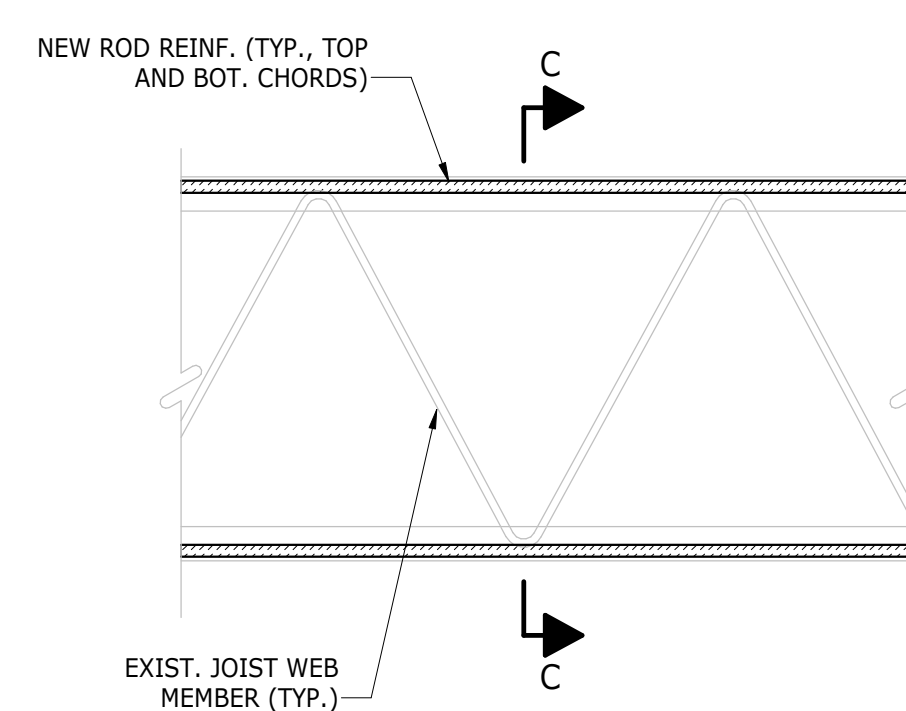


NOTES:

- REFER TO EXISTING JOIST ELEVATIONS FOR LIMITS OF WEB SHEAR RETROFIT.
- ANGLE REINFORCING SHALL HAVE A MINIMUM YIELD STRENGTH (F_y) OF 36 ksi.
- MINIMUM LENGTH OF WELD AT EA. END OF EACH ANGLE SHALL BE 2" LONG.

JOIST WEB RETROFIT DETAILS

NOT TO SCALE



WELD NOTES:

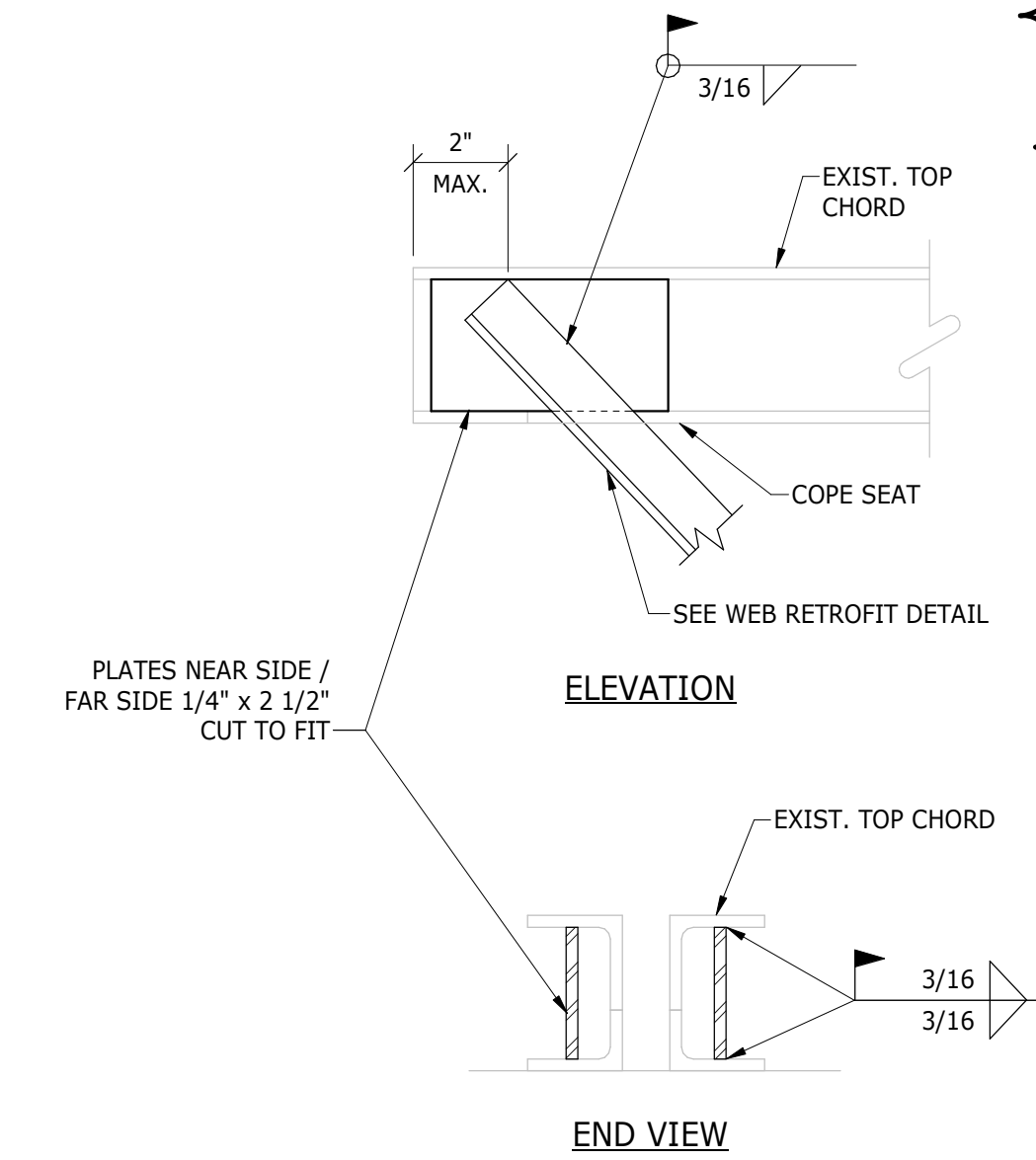
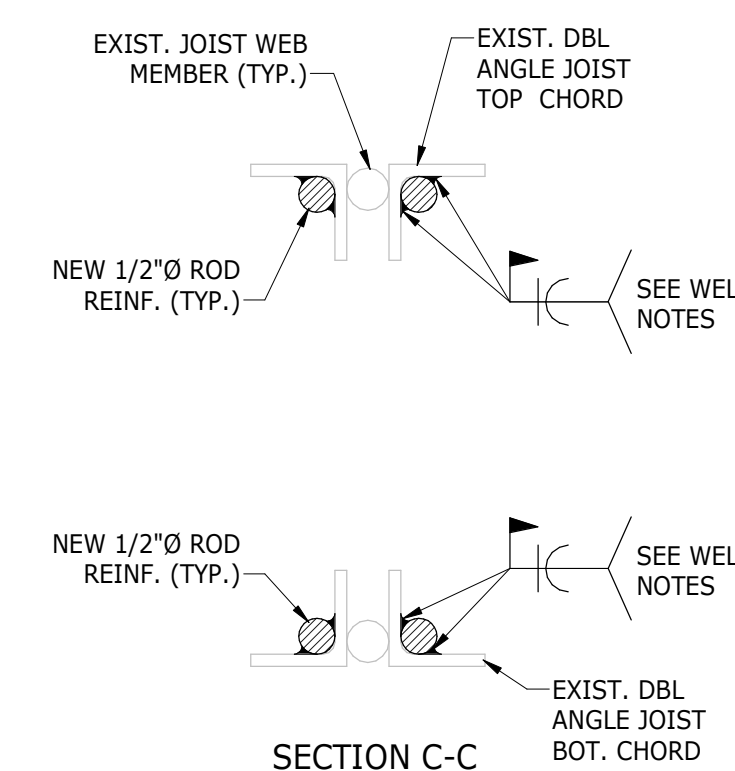
- 2" MINIMUM WELD AT ALL PANEL POINTS (TOP & BOTTOM).
- STITCH WELD 2" @ MIDPOINT BETWEEN PANEL POINTS.
- 6" LG., CONT. @ EA. END OF ROD.

NOTES:

- REFER TO EXISTING JOIST ELEVATIONS FOR LIMITS OF CHORD RETROFIT.
- ROD REINFORCING SHALL HAVE A MINIMUM YIELD STRENGTH (F_y) OF 36 ksi.

JOIST CHORD RETROFIT DETAILS

NOT TO SCALE



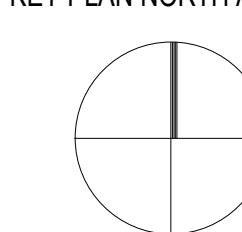
JOIST END SEAT RETROFIT DETAIL

NOT TO SCALE

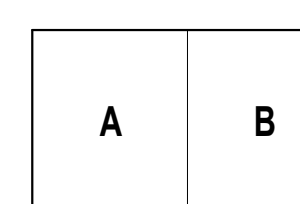
ADDENDUM #1 ADD-1 9.25.2023

ADDENDUMIZED DOCUMENTS

KEY PLAN NORTH ARROW



KEYPLAN



DRAWING NAME:

SECTIONS & DETAILS

DRAWN BY: JDB

REVIEWED BY: KMC

SCALE: AS INDICATED DRAWING NUMBER:

JOB NO.: 1903.03

DATE: September 18, 2023

S2.10